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### Signal optimization using Highway Capacity Software (HCS) 2010



### Overview and demonstration By Ishant Sharma (isharma@memphis.edu) Graduate Research Assistant Supervisor: Dr. Sabya Mishra



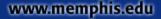
# Highway Capacity Software

### Developed and maintained by McTrans

- Originally founded by the FHWA in 1986 as the Center for Microcomputers in Transportation (McTrans)
- $\circ~$  Now a full-service software support center, associated with the University of Florida

### HCS 2010

- HCS 2010 implements the procedures defined in the Highway Capacity Manual (HCM) 2010 published by the Transportation Research Board (TRB)
- This release includes the new Streets module that combines the Signalized Intersections with the Urban Streets Segments, Facilities and Multimodal procedures.



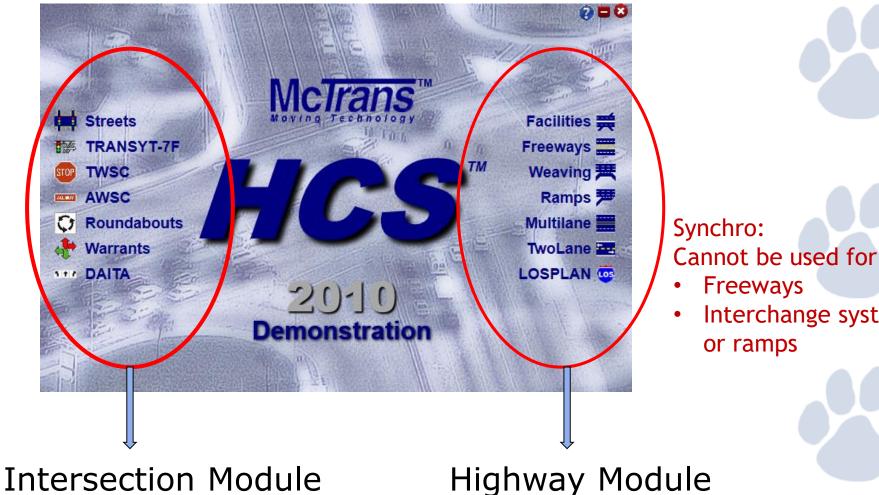
Freeways

or ramps

Interchange systems

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### **Fourteen Modules**





### **Street Module**

Signalized Intersections

- Signal analysis
- Interchange analysis
- Multimodal analysis
- LOS





## **Signalized intersections**

- Traffic Conditions
  - Approach volumes (left, through, right)
  - Vehicle type (heavy vehicle, bicycles)
  - Pedestrian movement

#### Dreamers. Thinkers. Doers.

## Signalized intersections

- Roadway Conditions
  - Number and width of lanes
  - Grades
  - Lane use
- Traffic Signal Characteristics
  - Signal phasing
  - Signal timing
  - Type of control (Actuated/pre-timed)
  - Signal progression (un/co-ordinated)





### Signalized intersections

### Total delay:

- Difference between actual travel time and ideal travel time
  - In the absence of traffic control, delay due to roadway geometries, incidents and when there are no vehicles on the road
- In HCS control delay is quantified
  - initial deceleration delay
  - Queue move-up time
  - Stopped delay
  - Final acceleration delay



## Level of Service (LOS)

- LOS criteria are stated in terms of average control delay per vehicle
  - Delay on signal control depends on
    - Quality of progression
    - Cycle length
    - Green ratio
    - V/c ratio for lane group
    - ICU (Intersection Capacity Utilization)
  - Designated by letters A F



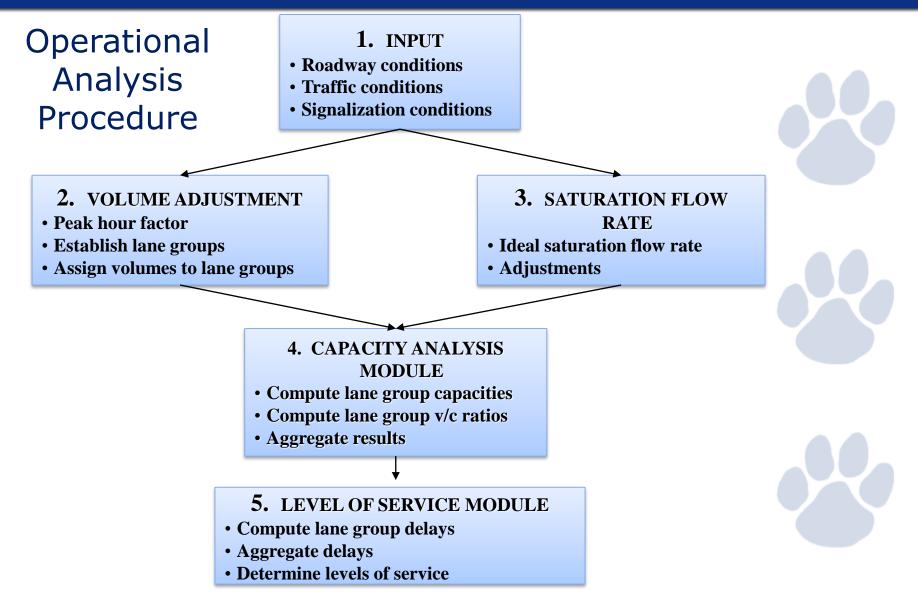
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## Level of Service (LOS)

LEVEL OF SERVICE	CONTROL DELAY PER VEHICLE (SEC/VEHICLES)
А	≤ 10
В	$>10 \text{ and } \le 20$
С	$>20$ and $\leq 35$
D	$>35$ and $\leq 55$
Е	$>55$ and $\leq 80$
F	> 80

ICU	Level of Service	
0 to 55%	Α	
>55% to 64%	В	
>64% to 73%	С	
>73% to 82%	D	
>82% to 91%	E	
>91% to 100%	F	
>100% to 109%	G	
>109%	Н	

Source: Highway Capacity Manual 2010





### **Getting Started:**

#### 1. Open HCS 2010

2. Select the Streets (handles signals and signalized corridors)





### THE UNIVERSITY OF MEMPHIS. Getting Started:

3. Complete The Quick Start Screen

Quick Start			
Default Selections			
Number of Intersections	1 🗘	Cycle Length, s	100
Forward Direction	NB 💌	Minimum Green, s	5
Number of Periods	1 🗘	Yellow Change, s	4.0
Analysis Duration, h	0.25	Red Clearance, s	1.0
Base Saturation Flow, pophpl	1900	Passage Time, s	2.0
Speed Limit, mi/h	45	Detector Length, ft	40
Template Help 🕜			Select Template
		<u>DK</u> ancel	







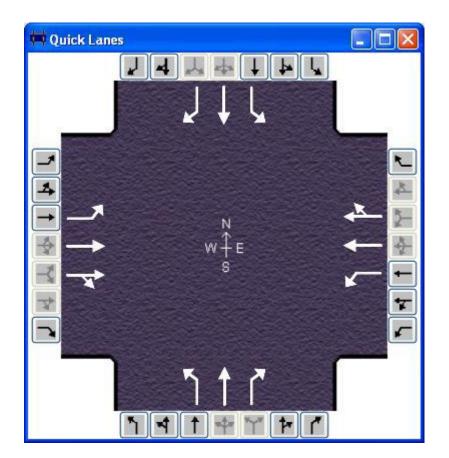
#### 4. Enter Information in the **General** Section

🖶 HCS 2010 Stree	ts - [Str	eet	s2.xus *]						
🖳 File View Ed	dit Win	Idow	s Repor	ts	Help				
i 🗅 📂 🖬 🖪 📗	🚦 Тлन 🔇	9	k 📖 🚺	7	7 🛑				
Classic Mode Visual	Mode								
General	DATA								
Urban Street	SR 42								
Intersection	SR 42 @	۶ Fo	rest Pkwy						
Description	Existing (	Existing Geometry Design Year Traffic							
Data File	Streets2.	xus							
Forward Direction	NB	•	Area Type	0	Other	•			
Segment Length, ft			Duration	0	).25				
All Segment L	engths		PHF	0	).92				





#### 5. Enter Lane Configuration





#### 6. Enter Traffic Data

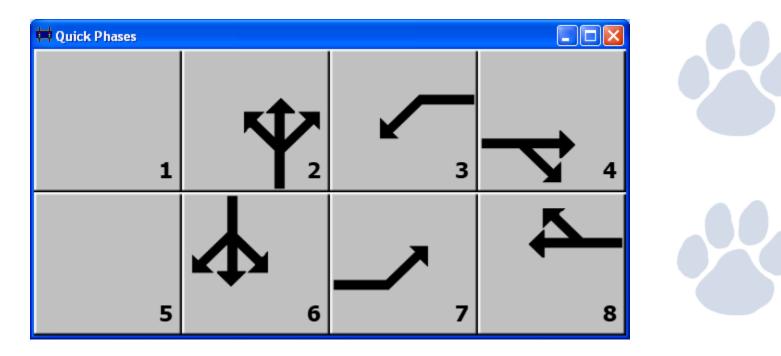
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Traffic	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Demand, veh/h	350	1315	285	385	740	125	130	320	240	225	780	310
Lane Width, ft	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0
Storage Length, ft	350	0	0	300	0	0	190	0	300	200	0	370
Saturation, pc/h/ln	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Heavy Vehicles, %	10	10	0	10	10	0	10	10	10	10	10	10
Grade, %		-2			1			6			-2	
Buses, per h			0			0			0			0
Parking, per h	0	Ν -	0	0	Ν.*	0	0	Ν.*	0	0	Ν.*	0
Bicycles, per h		0			0			0			0	
Pedestrians, per h		0			0			0			0	
Arrival Type	3	3	3	3	3	3	3	3	3	3	3	3
Upstream Filtering (I)		8	1.00		V8 )	1.00			1.00			1.00
Initial Queue, veh	0	0	0	0	0	0	0	0	0	0	0	0
Speed Limit, mi/h		45			45			45			45	
Detector, ft	40	40	40	40	40	40	40	40	40	40	40	40
RTOR, veh/h			0			0			0			0





#### 7. Enter Phasing data







#### Quick Exercise on Phase Diagram





#### 8. Enter **Timing**

Timing								
	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Phase Split, s	15.0	40.0	15.0	40.0	0.0	45.0	0.0	45.0
Yellow Change, s	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Red Clearance, s	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Minimum Green, s	5	5	5	5	5	5	5	5
Lag Phase	📃 EL	ET	wL	WT	NL	NT	SL	ST
Passage Time, s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Recall Mode	Off 🝷	Off 🝷	Off 🝷	Off 🝷	Off 🝷	Mir 💌	Off 🝷	Mir 💌
Dual Entry	EL	🖌 Et	🗌 WL	🗹 ул	NL	V NT	SL	🖌 ST
Dallas Phasing	E/W	N/S	S	imultane	ous Gap	🛃 EAW	🛃 N/S	

		~~~	∏7			
Green	40.0	10.0	35.0	0.0	0.0	0.0
Yellow	4.0	4.0	4.0	0.0	0.0	0.0
Red	1.0	1.0	1.0	0.0	0.0	0.0



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#### 9. Enter the **Detailed** Data

DETAILED INPUT D	ATA																		
General				Intersection -															
Analyst								EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Agency/Co			-	Lanes				1	2	1	1	2	1	1	2	0	1	2	0
Date	Wed, September 07,	2011	₹1	Shared Lane				<b>[</b> ]								V			V
Time Period			1	Percent Turns	in Shared	d Lane		0	]	0	0		0	0	]	0	0		0
Analysis Year			1	Percent Unop	posed Let	ft Turns	5	0	]		0			0			0		
Jurisdiction			-1	Heaviest Lane	Volume,	veh/h		0	500	0	0	500	0	0	0	0	0	0	0
				Start-Up Lost	Time, s			2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
General			_	Extension of E	ffective G	ireen, s	8	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Number of Calculation It	erations	15		Walk Interval,	5				0.0	]		0.0			0.0			0.0	
Critical Merge Gap, s		3.70		Pedestrian Cle	ar Interva	al, s			0.0	]		0.0			0.0			0.0	
Stored Vehicle Lane Le	ngth, ft	25.0		Receiving Lan	ies				2	]		2			2			2	
Length of Detected Veh	iicle, ft	17.0		Heavy Vehicle	Equivale	ency Fa	ctor	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Stored Heavy Vehicle L	ength, ft	45		Bus Blockage	Time, s					14.4	]		14.4			14.4			14.4
Queue Length Percentil	e	50	-	Parking Maner	uver Time	e, s		18	]	18	18		18	18		18	18		18
Acceleration Rate, ft/s2	2	3.50		Opposing Righ	it-Tum La	ne Influ	uence												
Stop Threshold Speed,	mi/h	5.0									-								
Pedestrians Pushing Bu	tton, prop	0.65		Segment		Γ				וה	- Signa							_	
Speed Limit to Base FF	5 Ratio	0.90		INGILIE			EB	WB Exclusive Pedestrian Phase Time, s				S		0.0					
Sneakers per Cycle, vel	ı	2.0		Upstream Wid	th, ft	E	50	50			Right-Turn Equivalency Factor 1.18						8		
Platoon Minimum Heady	vay, s/veh	1.50		Restrictive Me	dian, ft	C	)	0		ίIJ	Left-Tu	ım Equ	ivalenc	cy Fact	or			1.0	5
Platoon Maximum Head	way, s/veh	3.60		Right-Hand Cu	urb, %	7	70	70		i I I	Decele	ration	Rate, ft	t/s2				4.0	D
Platoon Dispersion Fact	or	0.138		- Right-Hand Ad	cess Poir	nts 4	ļ.	4		i I I	Critical	Heady	vay (pe	mitted	left tur	n), s		4.5	
Demand Growth, %	+ -	0	÷	Mid-Segment I	Delay, s∕\	/eh 🕻	).0	0.0		<u> </u>	Follow	-Up He	adway	(permit	ted lef	t tum),	5	2.5	
Access Point			Acc	ess Points															
Critical Headway (left fr	om major), s 4.	1	Ac	tive		٦													_
Follow-Up Headway (le	ft from major), s 2.	2	PH	IF 1.00	-	1>				•	→	_	unt: 2		Ne			Delete	
Right-Turn Equivalency	r Factor 2.	20			EBL	EBT		WB			WBR	NBL			IBR	SBL	SBT		BR
Maximum Turn Bay Ler	igth, ft 25	50	De	mand, veh/h	80	1050		80			100	80	0			80	0	10	0
Deceleration Rate, ft/s	2 6.	7	La	nes	0	2	0	0	2	(	-	1	0	1		1	0	1	
Right-Turn Speed, ft/s	20	)	Na	ime							Loca	tion, ft	600						





### 10. Run Full Optimization

8 Full Optimization		
<ul> <li>✓ Input Parameters</li> <li>Global Optimization</li> <li>Objective Function</li> <li>Overall Delay</li> <li>Cycle Length</li> <li>Phasing Sequence</li> <li>✓ Dallas Phasing</li> <li>Offsets</li> <li>✓ Optimization Status</li> <li>✓ Diagnostic Messages</li> </ul>	Minimum Cycle, s       60       Number of Generations       50         Maximum Cycle, s       120       Population Size       10         Cycle Increment, s       10       Crossover Probability, %       30         Master Intersection       1       Mutation Probability, %       4.0         Forward Weighting, %       50       Convergence Threshold, %       0.010         Reverse Weighting, %       50       Random Number Seed       7781	Start Info Stop
	Save Cancel	



#### 11. Optimization Results

Full Optimization					X
<ul> <li>Optimization Status</li> </ul>					<u>^</u>
	Overa	ll Delay			
Original		263.7 sec/v	reh		
Optimun	n	169.1 sec/\	/eh		
Average		175.0 sec/\	veh		
Improve	ment	35.9%			
	Run	Status			
Generat	ion Number	200 out of 2	200		
Generat	ion Optimum	101			
Total Tir	ne Elapsed	43 sec			
Termina	tion via max n	umber of gen	erations		
Diagnostic Messages					
					Start
No messages to report at th	is time.				Info     Stop
					Stop
		(	Save	Cancel	~



#### 12. View/Print Results Summary Report

HCS 2010 Signalized Intersection Results Summary

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General Informat	4:							1.	ntersec		l a f					
		GDOT							ntersec )uration		Int	0.25	n	- 1	JIC	
Agency						1						0.25 Other		-		
Analyst		Design Engineer				Apr 12			Area Type					1 1 P		-
Jurisdiction		Clayton County		Time Period 2032 PM				PHF			0.92		14			
Intersection		SR 42 @ Forest Pk	wy	Analys	sis Year	2012		A	nalysis	Peri	od	1> 7:0	00	-		
File Name		pdt example.xus													1.4	
Project Descriptio	on	Existing Geometry [	Design `	Year Tra	affic										R . O .	N R
Demand Informa	ation				EB			WB		T.		NB			SB	
Approach Movem	nent			L	Т	R	L .	Т	R	т	L	ΤT	R	L .	Т	T
Demand (v), veh/l	/h			350	1315	285	385	740	125	1	30	320	240	225	780	3
									÷	1		<u> </u>				
Signal Informatio														rta		
		Reference Phase	2		I	I							4	Y		7
Offset, s	0	Reference Point	End	Green		0.0	0.0	0.0	0.0		).0				i	2
	No	Simult. Gap E/W	On	Yellow		0.0	0.0	0.0	0.0		0.0		_ <b> </b> 4	P _	~	
Force Mode F	Fixed	Simult. Gap N/S	On	Red	0.0	0.0	0.0	0.0	0.0		0.0	_	5	4	7	
Timer Results				EBL		EBT	WB		WBT	1	NBI		NBT	SBI		SB
Assigned Phase				7		4	3		8		_		2			6
Case Number				2.0		4.0	2.0		4.0		_		5.0			5.0
Phase Duration, s	s			32.2	2 1	50.0	21.0	)	38.8		-		39.0			39.0
Change Period, (	Y+R₂).	5		5.0		5.0	5.0		5.0		_		5.0			5.0
Max Allow Headw			_	0.0		0.0	0.0		0.0		_	_	0.0		_	0.0
Queue Clearance				0.0	-	0.0	0.0	-	0.0		_	-	0.0			0.0
Green Extension			_	0.0		0.0	0.0		0.0		-		0.0			0.0
Phase Call Proba		3-71 -		0.00		0.00	0.00	)	0.00		_		0.00			0.00
Max Out Probabil			_	0.00		0.00	0.00		0.00		-		0.00			0.0
Movement Group	ıp Resi	ults			EB			WB				NB			SB	
Approach Movem	nent			L	Т	R	L	Т	R	L		Т	R	L	Т	
Assigned Movem	nent			7	4	14	3	8	18	5		2	12	1	6	1
Adjusted Flow Ra	ate (v),	veh/h		0	0	0	0	0	0	0		0	0	0	0	
Adjusted Saturation	ion Flo	w Rate (s), veh/h/ln		0	0	0	0	0	0	0		0	0	0	0	
Queue Service Ti	ime (g,	i), s		0.0	0.0	0.0	0.0	0.0	0.0	0.		0.0	0.0	0.0	0.0	0
Cycle Queue Clea	arance	Time (g:), s		0.0	0.0	0.0	0.0	0.0	0.0	0.		0.0	0.0	0.0	0.0	0
Capacity (c), veh/	/h			412	714	670	238	527	501	68		518	439	188	539	4
Volume-to-Capac	city Rat	tio (X)		0.924	1.233	1.282	1.758	0.915	0.915	2.1	9	0.672	0.594	1.303	1.572	0.1
Available Capacit	ty (ce),	veh/h		0	0	0	0	0	0	0		0	0	0	0	
Back of Queue (G	ସ), veh	/In (95th percentile)		15.2	58.8	61.9	47.3	21.1	20.3	21	В	13.5	10.4	22.9	82.4	13
Overflow Queue (	(Q3), V	eh/In		0.0	0.0	0.0	0.0	0.0	0.0	0.		0.0	0.0	0.0	0.0	0
	Ratio (F	RQ) (95th percentile)	)	1.17	0.00	0.00	4.25	0.00	0.00	3.0	9	0.00	0.94	3.09	0.00	1.
Queue Storage R		sh		40.4	32.5	32.5	47.0	38.7	36.7	55	D	33.1	32.2	50.4	38.0	34
Queue Storage R Uniform Delay (de	.,. s/ve			3.9	116.9	138.0	357.7	20.2	21.0	568	5	6.8	5.8	169.7	268.4	10
-	-	s/veh		0.0												
Uniform Delay (de	y (dz),			0.0	0.0	0.0	0.0	0.0	0.0	0		0.0	0.0	0.0	0.0	
Uniform Delay (de Incremental Delay	y (dz), av (d5)	s/veh			0.0 149.4	0.0	0.0 404.7	0.0 56.9	0.0 57.7	623	-	0.0 39.9	0.0 38.0	0.0 220.1	0.0 304.4	_
Uniform Delay (de Incremental Delay Initial Queue Dela	y (dz), sv (dz) , s/veh	s/veh		0.0						_	3.5					0 44



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#### 13. View Messages Report

- Look for any warnings

--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ----

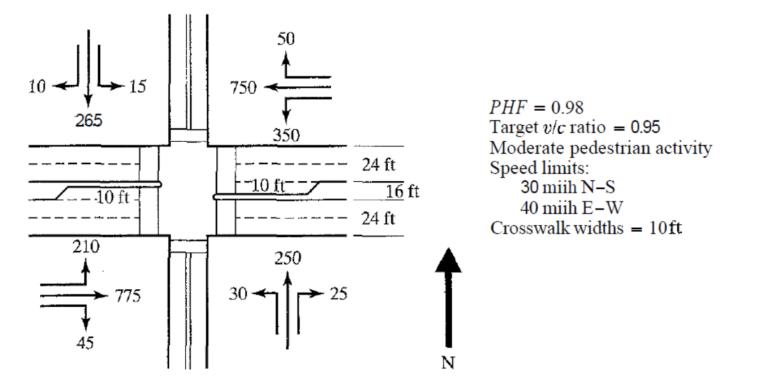






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2. What should be your recommended phasing and cycle length for the following intersection?





# **Questions?**

#### References

Mctrans Center (2016, May 7), HCS 2010 Streets - Tutorial [Video file]. Retrieved from <a href="https://www.youtube.com/watch?v=OjgB-ufDPfk">https://www.youtube.com/watch?v=OjgB-ufDPfk</a>

GDOT (2013), Getting Started: HCS 2010, retrieved from: http://www.dot.ga.gov/PartnerSmart/DesignManuals/ElectronicPlanProcess/Getting% 20Started%20HCS2010.pdf

